FreeFem++ Lessons 5-8

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Lesson 5 Freefem+

sticity and Non Linear Problems

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1 Linear Elasticity: weak formulations and programs (Lesson 5)

- 2 Non-linear problems (Lesson 6)
- 3 Mesh adaptation (Lesson 6)
- Incompressible Fluid Dynamics (Lesson 7)
- 5 Moving boundaries/ Eigenvalue problems / Parallel computing (Lesson 8)



Linear Elasticity: weak formulations and programs (Lesson 5)

- Linear elasticity equations
- Static Linear Lamé equation, weak formulation
- Representation of the Strain and Stress tensors
- Solving the static linear elasticity equation in 2d with FreeFem++
- Solving the static linear elasticity equation in 3d with FreeFem++
- Solving the time-dependent linear elasticity equation in 2d and 3d



Linear Lamé equation and Hooke's Law

Let us consider a beam and with transverse section Ω , subject to a force \mathbf{f} , perpendicular to the axis. The components along x and y of the displacement $\mathbf{u}(x)$ in the section Ω are governed by the Lamé's system of linear equations.

Remark: we do not use this equation because the associated variational form does not give the correct boundary conditions! We simply use the equilibrium between efforts and constraints:

 $-\nabla.(\sigma)=\rho \boldsymbol{f}\quad\text{in}\quad\Omega,$

where the constraint tensor $\sigma(u)$ is related to deformations using the Hooke's law:

 $\sigma(\boldsymbol{u}) = \lambda tr(\varepsilon(\boldsymbol{u}))I + 2\mu\varepsilon(\boldsymbol{u}).$

 λ, μ are the physical Lamé coefficients and the strain tensor is $\varepsilon(\boldsymbol{u}) = \frac{1}{2}(\nabla \boldsymbol{u} + {}^t\nabla \boldsymbol{u})$. The corresponding variational (weak) form is:

$$\int_{\Omega} \sigma(\boldsymbol{u}) : \varepsilon(\boldsymbol{v}) \ dx - \int_{\Omega} \boldsymbol{f} \boldsymbol{v} \ dx - \int_{\partial \Omega} \left(\sigma(\boldsymbol{u}) . \boldsymbol{n} \right) \boldsymbol{v} = 0, \qquad \boldsymbol{a} : \boldsymbol{b} = \sum_{i,j} a_{ij} b_{ij}.$$

Finally, the variational form can be written as :

$$\int_{\Omega} \lambda \, \nabla . \boldsymbol{v} \nabla . \boldsymbol{u} + 2\mu \, \varepsilon(\boldsymbol{u}) : \varepsilon(\boldsymbol{v}) \, d\boldsymbol{x} - \int_{\Omega} \boldsymbol{f} \boldsymbol{v} \, d\boldsymbol{x} - \int_{\partial \Omega} \left(\sigma(\boldsymbol{u}) . \boldsymbol{n} \right) \boldsymbol{v} = 0.$$

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Static Linear Lamé equations: weak formulation

Let $\Omega \subset \mathbb{R}^d$ be a domain with a partition of $\partial \Omega = \Gamma_d \cup \Gamma_n$. Find \boldsymbol{u} , the displacement field, such that:

 $-\nabla \sigma(\boldsymbol{u}) = \rho \boldsymbol{f} \text{ in } \Omega, \quad \boldsymbol{u} = \boldsymbol{0} \text{ on } \Gamma_d, \quad \sigma(\boldsymbol{u}) \boldsymbol{n} = 0 \text{ on } \Gamma_n, \tag{1}$

where $\sigma(\boldsymbol{u}) = \mathcal{A}\varepsilon(\boldsymbol{u})$, with \mathcal{A} a linear positive operator (symmetric $d \times d$ matrix) corresponding to material properties. Let us denote $V_{\boldsymbol{g}} = \{\boldsymbol{v} \in H^1(\Omega)^d / \boldsymbol{v}_{|\Gamma_d} = \boldsymbol{g}\}$. The basic (displacement) variational formulation is: find $\boldsymbol{u} \in V_0(\Omega)$, such that:

$$\int_{\Omega} \varepsilon(\boldsymbol{v}) : \mathcal{A}\varepsilon(\boldsymbol{u}) = \int_{\Omega} \rho \, \boldsymbol{v} \cdot \boldsymbol{f} + \int_{\Gamma} ((\mathcal{A}\varepsilon(\boldsymbol{u}))\boldsymbol{n}) \cdot \boldsymbol{v}, \quad \forall \boldsymbol{v} \in V_0(\Omega).$$
(2)

The Hooke's law says that $\mathcal{A} = \lambda \mathbf{I}_d + 2\mu \mathbf{1}_{d,d}$, where \mathbf{I}_d is the Identity $d \times d$ matrix and $\mathbf{1}_{d,d}$ the constant $d \times d$ matrix filled with 1. Question: How to code this equation with FreeFem++? <u>Remark:</u> the contraction operator (:) exists, but its priority is low: try to avoid it!



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Representation of the Strain tensor ε : the Voigt notation

```
More details on https://en.wikipedia.org/wiki/Voigt_notation
We denote by lambda = \lambda, mu = \mu, twomul = 2\lambda + \mu, and define
In 2d:
func A = [[twomul,lambda, 0. ],
[lambda,twomul, 0. ],
[ 0. , 0. , mu ]];
macro epsV(u1,u2) [dx(u1),dy(u2),dy(u1)+dx(u2)] // EOM
```

macro div(u1,u2) (dx(u1)+dy(u2)) // EOM

In 3d:



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Solving the static linear elasticity equation in 2d with FreeFem++

For the values of material constants, see

http://www.mstrtech.com/WebPages/matexam.htm

```
// Steel \lambda = 9.695 \ 10^{10} N/m^2,
// \mu = 7.617 \ 10^{10} N/m^2, \rho = 7700 kg/m^3.
real rho = 7700, mu = 7.617e10, lambda = 9.69e10;
real gravity = -9.81, twomul=2*mu+lambda; // Optimisation
cout << "lambda,mu,gravity_="<<lambda<< "..." << mu << "..." << gravity << endl;</pre>
The FreeFem++ code:
int[int] labs=[1,1,1,2];
mesh Th=square(50,5,[x*10,y],label=labs);
fespace Vh(Th, [P1, P1]);
Vh [u1,u2], [v1,v2], [un1,un2], [up1,up2];
solve Lame([u1,u2],[v1,v2]) = int2d(Th)( epsV(u1,u2)'*A*epsV(v1,v2))
  - int2d(Th) ( rho*gravity*v2) + on(2,u1=0,u2=0) ;
real dmax= u1[].linfty, coef= 3/dmax;
cout << ".max.deplacement, = ," << dmax << ".coef, " << coef << endl;</pre>
mesh Thm = change(movemesh(Th, [x+u1*coef, y+u2*coef]), freqion=1);
```

```
plot(Th, Thm, wait=1, cmm="coef__amplification_=_"+coef);
```

Run:Beam-Static-2d.edp



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Solving the static linear elasticity equation in 3d with FreeFem++

Using the same physical parameters: $\lambda=9.695\;10^{10}N/m^2$, $\mu=7.617\;10^{10}N/m^2$, $\rho=7700kg/m^3.$ The FreeFem++ code:

```
int[int] labs=[1,1,1,2,1,1];
mesh3 Th=cube(50,5,5,[x*10,y,z],label=labs);
```

Run:Beam-Static-3d.edp



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Solving the time-dependent linear elasticity equation in 2d and 3d

The problem is (strong formulation)

$$\rho \partial_{tt} \boldsymbol{u} - \nabla . (\sigma(\boldsymbol{u})) = \rho \boldsymbol{f} \quad \text{in} \quad \Omega.$$

We use a classical explicit 2nd order finite difference scheme for the time derivative:

$$\rho \frac{u^{n+1} - 2u^n + u^{n-1}}{(\delta t)^2} - \nabla . (\sigma(\boldsymbol{u}^n)) = \rho \boldsymbol{f}^n$$

Let us denote by $un = u^{n+1}$, $u = u^n$, $up = u^{n-1}$; the matrix formulation of the problem is:

$$un = M^{-1}b, \quad b = M(-up) + Au + r, \quad +B.C$$

$$\boldsymbol{M} \equiv \int_{\Omega} \rho \frac{\boldsymbol{u}.\boldsymbol{v}}{(\delta t)^2} + B.C, \quad \boldsymbol{A} \equiv \int_{\Omega} -\varepsilon(\boldsymbol{v}) : \mathcal{A}\varepsilon(\boldsymbol{u}) + 2\rho \frac{\boldsymbol{u}.\boldsymbol{v}}{(\delta t)^2}, \quad r \equiv \int_{\Omega} \rho \ \boldsymbol{g} \ \boldsymbol{e}_3.\boldsymbol{v} + B.C$$



Program for the time-dependent linear elasticity equation in 2d and 3d

```
include "Beam-Static-2d.edp"
real dt =1e-5, rhodt2= rho/dt/dt;
varf vA( [u1,u2],[v1,v2]) = int2d(Th)( -1*epsV(u1,u2)'*A*epsV(v1,v2)
               + 2*rhodt2*[u1,u2]'*[v1,v2]);
varf vM( [u1,u2],[v1,v2]) = int2d(Th)( rhodt2*[u1,u2]'*[v1,v2])
               + on(2,u1=0,u2=0);
varf vB( [u1,u2],[v1,v2]) = int2d(Th)( rho*[0,qravity]'*[v1,v2])
               + on(2,u1=0,u2=0);
matrix AA=vA(Vh,Vh), M=vM(Vh,Vh,solver=CG);
real[int] Rhs = vB(0,Vh);
func BB=[[-0.5, -7], [10.5, 1.4]]; // for fixe bounding box of the plot ...
up1[]=u1[]=0;
for(int i=0; i<100000; ++i) {</pre>
    real[int] b = AA*u1[]; up1[]=-up1[]; b += Rhs; b += M*up1[];
    un1[] = M^{-1}*b;
    up1[]=u1[]; u1[]=un1[];
    if(i%100==0) { cout << i << "." << u1[].linfty << endl;
    mesh Thmm =movemesh(Th, [x+u1*coef, v+u2*coef]);
```

Run:Beam-Vibration2d.edp

Run:Beam-Vibration3d.edp



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2 Non-linear problems (Lesson 6)

- Algorithms for solving non-linear problems: fixed point algorithm, Newton method
- Example of a non-linear problem: the Minimal Surface problem
- A fixed-point method to solve the Minimal Surface problem
- A Newton method to solve the Minimal Surface problem



The fixed-point algorithm

Consider the non-linear problem F(u, u) = 0, with F(., u) affine with respect to the first variable. To find a solution, you can try the following basic method, with no guaranty of convergence:

- set u^0 an initial guess
- **2** do (iterations following n)
 - find u^{n+1} , the solution to $F(u^{n+1}, u^n) = 0$,
 - 2 if $(||u^{n+1} u^n|| < \varepsilon)$ break;

The difficulty in this algorithm is to find an initial guess; sometimes this algorithm explodes. The convergence is generally slow.



The Newton method

To solve F(u) = 0 we can also use the Newton method (DF is the differential of F):

- **1** set u^0 , an initial guess
- **2** do (iterations following n)
 - find w^n , solution to $DF(u^n)w^n = F(u^n)$
 - **2** $u^{n+1} = u^n w^n$
 - $\ \, \hbox{ if} (\ ||w^n|| < \varepsilon) \ \hbox{break};$

The Optimized Newton Method:

if F = C + L + N, with C the constant, L the linear, and N the non-linear part of F. We obtain that DF = L + DN and the Newton method can be written as: $DF(u^n)u^{n+1} = DF(u^n)u^n - F(u^n) = DN(u^n)u^n - N(u^n) - C$. The new version of the algorithm is:

🚺 do

 $\begin{array}{ll} \bullet & \mbox{find } u^{n+1} \mbox{ solution to } \\ DF(u^n)u^{n+1} & = DN(u^n)u^n - N(u^n) - C \\ \bullet & \mbox{if(} ||u^{n+1} - u^n|| < \varepsilon) \mbox{ break;} \end{array}$

The weakness of this algorithm is the need to start from an initial guess sufficiently close to a solution.



2 Non-linear problems (Lesson 6)

- Algorithms for solving non-linear problems: fixed point algorithm, Newton method
- Example of a non-linear problem: the Minimal Surface problem
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JDWC

Example of a non-linear problem: the Minimal Surface problem

Let us solve the following geometrical problem: Find a function $u: \Omega \mapsto \mathbb{R}$, where u is given on $\Gamma = \partial \Omega$, (i.e. $u_{|\Gamma} = g$) such as the area of the surface S, parametrized by $(x, y) \in \Omega \mapsto (x, y, u(x, y))$ is minimal. The mathematical formulation of the problem is:

$$\arg\min J(u) = \int_{\Omega} \left\| \begin{pmatrix} 1\\ 0\\ \partial_x u \end{pmatrix} \times \begin{pmatrix} 0\\ 1\\ \partial_y u \end{pmatrix} \right\| d\Omega = \int_{\Omega} \sqrt{1 + (\partial_x u)^2 + (\partial_y u)^2} \, d\Omega.$$

The Euler-Lagrange equation associated to the minimization of J(u) is:

$$\forall v/v_{|\Gamma} = 0 \quad : \quad DJ(u)v = \int_{\Omega} \frac{(\partial_x v \partial_x u + \partial_y v \partial_y u)}{\sqrt{1 + (\partial_x u)^2 + (\partial_y u)^2}} \ d\Omega = 0.$$

We consider the case: $\Omega =]0, \pi[^2 \text{ and } g(x, y) = \cos(nx)\cos(ny),$ n = 1 (simplest problem) and n = 2 or 4 (harder to solve). We shall use the fixed-point algorithm and the Newton method.



2 Non-linear problems (Lesson 6)

- Algorithms for solving non-linear problems: fixed point algorithm, Newton method
- Example of a non-linear problem: the Minimal Surface problem

• A fixed-point method to solve the Minimal Surface problem

• A Newton method to solve the Minimal Surface problem



A fixed-point method to solve the Minimal Surface problem

```
int nn=100, n=4; // n= 1 ,2,4
int[int] 11=[1,1,1,1];
mesh Th= square(nn,nn,[x*pi,y*pi],label=11);
func q = \cos(n \star \mathbf{x}) \star \cos(n \star \mathbf{y});
fespace Vh(Th,P1);
Vh un, u, v;
for(int i=0; i< 1000; ++i)</pre>
    verbositv =0;
     solve Pb(un, v) = int2d(Th) ( (dx(un) *dx(v) + dy(un) *dy(v))
                         / \text{sgrt}(1. + (\mathbf{dx}(u) * \mathbf{dx}(u) + \mathbf{dy}(u) * \mathbf{dy}(u))))
     + on (1, un = q);
     real J = int2d(Th)(sqrt(1. + (dx(un) * dx(un) + dy(un) * dy(un))));
     plot (un, dim=3, fill=1, wait=0);
     u[]-=un[]; // diff
     real err= u[].linfty;
     cout << "_iter_" << i << "_" << err <<"_" << "_J_" << J << endl;</pre>
     if( err < 1e-6) break;</pre>
     u[]=un[]; }
```

Run:Min-Surf-FixPoint.edp



2 Non-linear problems (Lesson 6)

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A Newton method to solve the Minimal Surface problem

```
// macro of compute all differentiel
macro grad2(u,v) ( dx(u) * dx(v) + dy(u) * dy(v) ) //
macro JJ(u) sqrt( 1. + grad2(u,u) ) //
macro dJJ(u,du) ( grad2(u,du) / JJ(u) ) //
macro ddJJ(u,du,ddu) ( grad2(ddu,du)/JJ(u)
                   - (grad2(u,du)*grad2(u,ddu)/JJ(u)^3) ) // For Newton
fespace Vh(Th,P1);
Vh u,v,w;
// Stating point ...
solve Pb0(u,v) = int2d(Th)(grad2(u,v)) + on(1, u = g);
plot(u,dim=3,wait=0);
// Newton loop
for(int i=0; i< 100; ++i)</pre>
{ verbositv =0;
    solve PbTangent(w,v) = int2d(Th)( ddJJ(u,w,v) ) - int2d(Th)( dJJ(u,v) )
    + on (1, 2, 3, 4, w = 0);
    u[] -=w[];
    real J = int2d(Th)(JJ(u));
    plot (u, dim=3, fill=1, wait=0, cmm="..J.="+J);
    real err= w[].linfty;
    cout << "_iter_" << i << "_err=_" << err <<"_" << "_J." << J << end];</pre>
    if( err < 1e-6 || err >100) break; }
```

Run:Min-Surf-Newton.edp

Run:Min-Surf-Newton-V2.edp

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3 Mesh adaptation (Lesson 6)

- Metrics and Unit Mesh
- Metrics and norms
- Solving the 2d Poisson equation using mesh adaptation
- Solving the 3d Poisson equation using mesh adaptation
- A Newton method with mesh adaptation for the Minimal Surface problem



Mesh adaptivity: Metrics and Unit Mesh

In Euclidean geometry the length $|\gamma|$ of a curve γ of \mathbb{R}^d parametrized by $\gamma(t)_{t=0..1}$ is

$$\gamma| = \int_0^1 \sqrt{<\gamma'(t),\gamma'(t)>} dt$$

We introduce the metric $\mathcal{M}(x)$ as a field of $d \times d$ symmetric positive definite matrices, and the length ℓ of Γ w.r.t \mathcal{M} is:

$$\ell = \int_0^1 \sqrt{\langle \gamma'(t), \mathcal{M}(\gamma(t))\gamma'(t) \rangle} dt$$

The key-idea is to construct a mesh for which the lengths of the edges are close to 1, accordingly to \mathcal{M} .



For a metric \mathcal{M} , the unit ball \mathcal{BM} (obtained by plotting the maximum mesh size in all directions), is a ellipse.

If you we have two unknowns u and v, we just compute the metrics \mathcal{M}_u and \mathcal{M}_v , find a metric \mathcal{M}_{uv} , called intersection, defined by the biggest ellipse such that:

 $\mathcal{B}(\mathcal{M}_v) \subset \mathcal{B}(\mathcal{M}_u) \cap \mathcal{B}(\mathcal{M}_v)$





Example of an adaptive mesh

$$u = (10x^{3} + y^{3}) + tanh(500(sin(5y) - 2x)));$$

$$v = (10y^3 + x^3) + tanh(5000(sin(5y) - 2*)));$$

Run:Adapt-uv.edp





3 Mesh adaptation (Lesson 6)

• Metrics and Unit Mesh

Metrics and norms

- Solving the 2d Poisson equation using mesh adaptation
- Solving the 3d Poisson equation using mesh adaptation
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Building the metrics from the solution u

For P_1 continuous Lagrange finite elements, the optimal metric norms for the interpolation error (used in the function adaptmesh in FreeFem++) are:

- L^{∞} : $\mathcal{M} = \frac{1}{\varepsilon} |\nabla \nabla u| = \frac{1}{\varepsilon} |\mathcal{H}|$, where $\mathcal{H} = \nabla \nabla u$
- L^p : $\mathcal{M} = \frac{1}{\varepsilon} |det(\mathcal{H})|^{\frac{1}{2p+2}} |\mathcal{H}|$, (result by F. Alauzet, A. Dervieux)

For the norm $W^{1,p}$, the optimal metric \mathcal{M}_{ℓ} for the P_{ℓ} Lagrange finite element is given by (with only acute triangles) (thanks to J-M. Mirebeau)

$$\mathcal{M}_{\ell,p} = rac{1}{arepsilon} (det \mathcal{M}_{\ell})^{rac{1}{\ell p+2}} \ \mathcal{M}_{\ell}$$

and (see MetricPk plugin and function)

• for P_1 : $\mathcal{M}_1 = \mathcal{H}^2$ (sub-optimal: for acute triangles, take \mathcal{H})

• for
$$P_2$$
: $\mathcal{M}_2 = 3\sqrt{\binom{a \ b}{b \ c}^2 + \binom{b \ c}{c \ a}^2}$ with
 $D^{(3)}u(x,y) = (ax^3 + 3bx^2y + 3cxy^2 + dy^3)/3!$

Run:adapt.edp



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The problem with a corner singularity (adaptivity with metrics)

The domain is a L-shaped polygon $\Omega=]0,1[^2\backslash[\frac{1}{2},1]^2$ and the PDE is

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find $u \in H_0^1(\Omega)$ such that $-\Delta u = 1$ in Ω .

The solution has a singularity at the re-entrant angle and we wish to capture it numerically.



The problem with a corner singularity (FreeFem++ program)

```
int[int] lab=[1,1,1,1];
mesh Th = square(6,6,label=lab);
Th=trunc(Th, x<0.5 | y<0.5, label=1);
fespace Vh(Th,P1);
Vh u,v;
real error=0.01;
problem Problem1(u,v,solver=CG,eps=1.0e-6) =
     int2d(Th)(dx(u)*dx(v) + dy(u)*dy(v)) - int2d(Th)(v)
     + on(1, u=0);
for (int i=0;i< 7;i++)</pre>
   Problem1;
                                      // solving the pde problem
   plot(u, Th, wait=1);
   Th=adaptmesh(Th, u, err=error, nbvx=100000); // the adaptation with Hessian of u
   u=u;
};
```

Run:CornerLap.edp





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Solving the 3d Poisson equation using mesh adaptation

```
load "msh3" load "tetgen" load "mshmet" load "medit"
int nn = 6; int[int] l1=[1,1,1,1,1,1];
mesh3 Th3=trunc( cube(nn,nn,nn,label=11)
                , (x<0.5) | (y < 0.5) | (z < 0.5), label=1);</pre>
fespace Vh(Th3,P1); Vh u,v,h;
macro Grad(u) [dx(u), dy(u), dz(u)] // EOM
problem Poisson(u,v,solver=CG) = int3d(Th3)(Grad(u)'*Grad(v))
       -int3d(Th3)(1*v) + on(1,u=0);
real errm=le-2;// level of error
for(int ii=0; ii<5; ii++)</pre>
{ Poisson;
  cout << "_u_min, _max_=_" << u[].min << "_"<< u[].max << endl;</pre>
  h=0. ;// for resizing h
  h[]=mshmet(Th3,u,normalization=1,aniso=0,nbrequl=1,hmin=1e-3,hmax=0.3,err=errm);
  cout << " h min, max = " << h[].min << "."<< h[].max << "." << h[].n << "."
       << Th3.nv << ",," << Th3.nt << endl;
  plot(u,wait=1);
  errm*= 0.8;// change the level of error
  Th3=tetgreconstruction (Th3, switch="raAO", sizeofvolume=h*h*h/6.); }
Poisson;
medit ("U-adap-iso-"+5, Th3, u, wait=1);
```

Run:Laplace-Adapt-3d.edp



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• A Newton method with mesh adaptation for the Minimal Surface problem



A Newton method with mesh adaptation for the Minimal Surface problem

```
real errA=0.1;
for(int adap=0; adap<7; adap++)</pre>
{ verbositv =0;
  for(int i=0; i< 100; ++i)</pre>
   { // ALGO NEWTOW OPTIMIZE
    solve PbTangent(un,v) = int2d(Th)( ddJJ(u,un,v) ) - int2d(Th)( ddJJ(u,u,v) -
        dJJ(u,v) )
    + on (1, 2, 3, 4, un = q);
    w[] =u[] -un[]; u[]=un[];
    real J = int2d(Th)(JJ(u));
    plot (u, dim=3, fill=1, wait=0, cmm="..J.="+J);
    real err= w[].linfty;
    cout << ".iter." << i << "." << err <<"." << ".J." << J << "." << "." << errA
         << endl;
    if( err < 1e-5) break;</pre>
    assert(err<10); }</pre>
  cout << "adaptmesh..." << endl;</pre>
  Th = adaptmesh(Th, u, err=errA, nbvx=100000, ratio = 1.5);
  plot(Th,WindowIndex=1);
  v=0;u=u; w=0; un=un; // resize
  errA = errA/2;
```

Run:Min-Surf-Newton-Adapt.edp



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- 2 Non-linear problems (Lesson 6)
- 3 Mesh adaptation (Lesson 6)
- Incompressible Fluid Dynamics (Lesson 7)
- 5 Moving boundaries/ Eigenvalue problems / Parallel computing (Lesson 8)



Incompressible Fluid Dynamics (Lesson 7)

• The stress tensor for a Newtonian fluid

- Stokes equation: variational formulation
- Incompressible Navier-Stokes equation: steady states



The stress tensor for a Newtonian fluid

In the domain Ω of \mathbb{R}^d , we denote by u the velocity field, p the pressure field and μ the dynamic viscosity. The classical mechanical stress σ^* of the fluid is:

$$\boldsymbol{\sigma}^{\star}(\boldsymbol{u},p) = 2\mu \mathbb{D}(\boldsymbol{u}) - p I_d, \qquad \mathbb{D}(\boldsymbol{u}) = \frac{1}{2} (\nabla \boldsymbol{u} + {}^t \nabla \boldsymbol{u})$$
(3)

or in the math formulation:

$$\boldsymbol{\sigma}^{\bullet}(\boldsymbol{u},p) = \mu \nabla \boldsymbol{u} - p I_d \tag{4}$$

So $\boldsymbol{\sigma}$ is one of these two stress tensors. Remark: if $\nabla \cdot \boldsymbol{u} = 0$ and μ is constant, then $\nabla \cdot 2\mathbb{D}(\boldsymbol{u}) = \mu \nabla \cdot \nabla \boldsymbol{u} + \mu \nabla \cdot \nabla \boldsymbol{u} = \mu \nabla \cdot \nabla \boldsymbol{u} + \mu \nabla \cdot \nabla \cdot \boldsymbol{u} = \mu \nabla^2 \boldsymbol{u} = \mu \Delta \boldsymbol{u}.$

Stokes Equation: find the velocity field u and the pressure field p, satisfying :

$$-\nabla . \boldsymbol{\sigma}(\boldsymbol{u}, p) = \boldsymbol{f} \quad (5) \quad \text{or} \quad -\mu \Delta \boldsymbol{u} + \nabla p = \boldsymbol{f} \quad (7) \\ -\nabla . \boldsymbol{u} = 0 \quad (6) \quad \nabla . \boldsymbol{u} = 0 \quad (8)$$

where f is the density of external forces.

+ Boundary conditions that will be defined through the variational (weak) form.



- Incompressible Fluid Dynamics (Lesson 7)
 - The stress tensor for a Newtonian fluid
 - Stokes equation: variational formulation
 - Incompressible Navier-Stokes equation: steady states



Stokes equation: variational formulation

Mechanical variational form of the Stokes equation:

$$\forall \boldsymbol{v}, q; \quad \int_{\Omega} 2\mu \mathbb{D}(\boldsymbol{u}) : \mathbb{D}(\boldsymbol{v}) - q\nabla \boldsymbol{.} \boldsymbol{u} - p\nabla \boldsymbol{.} \boldsymbol{v} = \int_{\Omega} \boldsymbol{f} \boldsymbol{.} \boldsymbol{v} + \int_{\Gamma} {}^{t} \boldsymbol{n} \boldsymbol{\sigma}^{\star}(\boldsymbol{u}, p) \boldsymbol{v}$$

Mathematical variational form of the Stokes equation:

$$\forall \boldsymbol{v}, q; \quad \int_{\Omega} \mu \nabla \boldsymbol{u} : \nabla \boldsymbol{v} - q \nabla \boldsymbol{u} - p \nabla \boldsymbol{v} = \int_{\Omega} \boldsymbol{f} \boldsymbol{v} + \int_{\Gamma} {}^{t} \boldsymbol{n} \boldsymbol{\sigma}^{\bullet}(\boldsymbol{u}, p) \boldsymbol{v}$$

But remember that ${}^tn\sigma^{\bullet}(u,p)$ are boundary density forces f_{Γ} and not ${}^tn\sigma^{\star}(u,p)$.

If the B.C. is $u = u_{\Gamma}$ for all boundaries, then the two formulations are identical. The pressure p is defined up to an additive constant and the weak formulation can use a small regularization (to remove the problem of the additive constant and impose a zero mean value for the pressure).

$$\forall \boldsymbol{v} \in (H_0^1)^d, q \in L^2; \quad \int_{\Omega} \mu \nabla \boldsymbol{u} : \nabla \boldsymbol{v} - q \nabla \boldsymbol{.} \boldsymbol{u} - p \nabla \boldsymbol{.} \boldsymbol{v} - \boldsymbol{\varepsilon} \boldsymbol{p} \boldsymbol{q} = \int_{\Omega} \boldsymbol{f} \boldsymbol{.} \boldsymbol{v}$$



Solving the Stokes equation with FreeFem++: the entrained cavity flow

```
int nn=10;
mesh Th=square(nn,nn);
macro grad(u) [dx(u),dy(u)] //
macro Grad(u1,u2) [grad(u1),grad(u2)] //
macro D(u1,u2) [ [dx(u1),(dy(u1)+dx(u2))*.5] , [(dy(u1)+dx(u2))*.5,dy(u2)] ] //
macro div(u1,u2) (dx(u1)+dy(u2))//
real epsp =1e-8, mu = 1;
```

Choose the correct finite-element couple for velocity and pressure: (P2,P1), (P1b,P1), (P1nc, P0), ...

fespace Vh(Th,P2); fespace Ph(Th,P1); // Taylor Hood Finite element

Run:Stokes-Cavity.edp



Incompressible Fluid Dynamics (Lesson 7)

- The stress tensor for a Newtonian fluid
- Stokes equation: variational formulation

• Incompressible Navier-Stokes equation: steady states



Incompressible Navier-Stokes equation: steady states

Computing steady states of the Incompressible Navier-Stokes equation: In the domain Ω of \mathbb{R}^d , find the velocity field u and the pressure field p, solution to:

$$(\boldsymbol{u}.\nabla)\boldsymbol{u} - \nabla.\boldsymbol{\sigma}(\boldsymbol{u},p) = \boldsymbol{f}, \qquad (9)$$

$$-\nabla \boldsymbol{u} = 0, \qquad (10)$$

+ Boundary conditions.

<u>First idea</u>: use the Optimized Newton Method (see page 19)! the only non-linear term is $N(u) = (u.\nabla)u$ and the differential is $DN(u)w = (u.\nabla)w + (w.\nabla)u$; so, the iteration ℓ of the Newton algorithm is: Find u^{ℓ}, p^{ℓ} such that

$$\begin{split} \forall \boldsymbol{v} \in (H_0^1)^d, & q \in L^2; \\ & \int_{\Omega} \mu(\nabla \boldsymbol{u}^{\ell} : \nabla \boldsymbol{v}) - q \nabla . \boldsymbol{u}^{\ell} - p^{\ell} \nabla . \boldsymbol{v} + \boldsymbol{v}.((\boldsymbol{u}^{\ell} . \nabla) \boldsymbol{u}^{\ell-1} + (\boldsymbol{u}^{\ell-1} . \nabla) \boldsymbol{u}^{\ell}) - \varepsilon p^{\ell} q \\ &= \int_{\Omega} \boldsymbol{v}.((\boldsymbol{u}^{\ell-1} . \nabla) \boldsymbol{u}^{\ell-1}) + \boldsymbol{f}. \boldsymbol{v} \end{split}$$

JDWC



Incompressible Navier-Stokes equation: steady states of the entrained cavity

```
real epsp =1e-8, mu = 1./Reynold , eps= 1e-5;
Vh u1=0,u2=0, un1,un2, v1,v2; Ph p,pn,q;
macro UGradW( u1,u2, w1,w2) [ [u1,u2]'*grad(w1) , [u1,u2]'*grad(w2)]//
verbosity=0;
for(int iter=0; iter<20; ++iter)</pre>
\{ // DF(u)un = DN(u)u - N(u) = UGradW(u1, u2, u1, u2) \}
   solve Tangent ([un1, un2, pn], [v1, v2, q]) =
     int2d(Th) ( UGradW(u1,u2, un1,un2)'*[v1,v2]
              + UGradW(un1, un2, u1, u2)' * [v1, v2]
              + mu*(Grad(un1, un2):Grad(v1, v2))
            - div(un1,un2)*q - div(v1,v2)*pn -epsp*pn*q
   - int2d(Th) ( UGradW(u1,u2,u1,u2)'*[v1,v2] )
 + on (1, 2, 4, un1=0, un2=0) + on (3, un1=1, un2=0) ;
   u1[]-=un1[]; u2[]-=un2[]; p[]-=pn[]; //diff err
   real err1=u1[].linfty, err2 =u2[].linfty , errp = p[].linfty;
   cout << " iter =" <<iter << " errs= " << err1 << " "<< err2 << " " << errp <<
       endl:
   u1[]=un1[]; u2[]=un2[]; p[]=pn[];
   plot([u1, u2], p, wait=1, cmm=iter);
   if( err1 < eps & err2 < eps & errp < eps) break;</pre>
```

Run:Navier-Stokes-Cavity.edp



Incompressible Navier-Stokes equation: unsteady computations

In the domain Ω of \mathbb{R}^d , find the velocity field u and the pressure field p, solution to:

$$\partial_t \boldsymbol{u} + (\boldsymbol{u} \cdot \nabla) \boldsymbol{u} - \nabla \cdot \boldsymbol{\sigma}(\boldsymbol{u}, p) = \boldsymbol{f},$$
 (11)

$$-\nabla . \boldsymbol{u} = 0, \qquad (12)$$

+ Initial conditions + Boundary conditions.

We try to compute the classical Benchmark: Computations of Laminar Flow Around a Cylinder form, by M. Schäfer, S. Turek, F. Durst, E. Krause, R. Rannacher

http://www.mathematik.tu-dortmund.de/lsiii/cms/papers/

SchaeferTurek1996.pdf We compute the 2d case.

The Geometry and the physical constant are defined in file Run:2d-data-Turek-bm.edp. One of the difficulties is to obtain the correct Strouhal number of the Bénard-von Karman vortex street.

We need a high-order scheme for the time integration: we use a multi-step BDF scheme of order 1, 2 or 3: BDF1 is Euler, BDF2 is $\partial_t u \sim \frac{3u^{n+1}-4u^n+u^{n-1}}{2\delta t}$ and BDF3 is $\partial_t u \sim \frac{11u^{n+1}-18u^n+9u^{n-1}-2u^{n-2}}{6\delta t}$ (https://en.wikipedia.org/wiki/Backward_differentiation_formula)



Incompressible Navier-Stokes equation: unsteady computations (program)

to empty the file

```
{ofstream ff(datafile); }// empty file ..
to write in a file.
```

```
to write in a file,
```

Run:NS-Newton-Turek-bm.edp



Unsteady incompressible Navier-Stokes equation: method of characteristics

For a flow field u the total (or material) derivative is

$$\frac{D\boldsymbol{u}}{Dt} = \frac{\partial \boldsymbol{u}}{\partial t} + (\boldsymbol{u} \cdot \nabla) \boldsymbol{u},$$

A correct numerical scheme used to approximate $\frac{Du}{Dt}$ has to take into account the movement of a particle: let us denote by x^n (resp. x^{n+1}) the particle position at time t^n (resp. t^{n+1}); we can write

$$\frac{D\boldsymbol{u}}{Dt}(x^{n+1}) \sim \frac{\boldsymbol{u}^{n+1}(x^{n+1}) - \boldsymbol{u}^n(x^n)}{\delta t}$$

Defining the characteristic flow (passing at time t through the point x)

$$\begin{cases} \frac{\partial \boldsymbol{X}}{\partial \tau}(\tau, t, \boldsymbol{x}) = \boldsymbol{u}(\tau, \boldsymbol{X}(\tau, t, \boldsymbol{x})), & \tau \in (0, t_{max}) \\ \boldsymbol{X}(t, t, \boldsymbol{x}) = \boldsymbol{x}, \end{cases}$$
(13)

one can express the total derivative of any function $\Phi(t, \boldsymbol{x})$ as

$$\frac{D\Phi}{Dt}(t,\boldsymbol{x}) = \left(\frac{\partial\Phi}{\partial t} + \boldsymbol{u}.\nabla\Phi\right)(t,\boldsymbol{x}) = \frac{\partial}{\partial t}\left(\Phi(\tau,\boldsymbol{X}(\tau,t,\boldsymbol{x}))\right)|_{\tau=t}$$
(14)

Fields Institute, Toronto, March 2016.

Method of characteristics in FreeFem++

We use the time discretization:

$$\left(\frac{D\Phi}{Dt}\right)^{n+1}(\boldsymbol{x}) \approx \frac{\Phi^{n+1}(\boldsymbol{x}) - \Phi^n \circ \boldsymbol{X}^n(\boldsymbol{x})}{\delta t},$$
(15)

with $X^n(x)$ a suitable approximation of $X(t_n, t_{n+1}, x)$, obtained by an integration back in time of (13) from t_{n+1} to t_n for each grid point x. The Galerkin characteristic method is implemented in Freefem++ as an operator computing $\Phi \circ X^n$ for given: mesh, convection velocity field and time step.

The FreeFem++ operator convect ([u1,u2],-dt, ..) computes:

$$\frac{D\boldsymbol{u}}{Dt}(x^{n+1}) \sim \frac{\boldsymbol{u}^{n+1} - \boldsymbol{u}^n \circ X^n}{\delta t}$$

Example: solve the convection equation with given velocity $oldsymbol{u}$

 $\partial_t a + (\boldsymbol{u} \cdot \nabla) a = 0, + \text{initial condition}$

- Linear Elasticity: weak formulations and programs (Lesson 5)
- 2 Non-linear problems (Lesson 6)
- 3 Mesh adaptation (Lesson 6)
- 4 Incompressible Fluid Dynamics (Lesson 7)
- 5 Moving boundaries/ Eigenvalue problems / Parallel computing (Lesson 8)



5 Moving boundaries/ Eigenvalue problems / Parallel computing (Lesson 8)

- \bullet A free-boundary problem: modelling the water infiltration 1/2
- Eigenvalue problems



A free-boundary problem: modelling the water infiltration

We use a simple model to study water infiltration = the process by which water on the ground surface enters the soil.

```
Let \Omega be a trapezoidal domain, defined in <code>FreeFem++</code> by:
```

```
real L=10,h=2.1 h1=0.35; //Lenght, Left and Right Height
// trapeze
border a(t=0,L) {x=t;y=0;}; // bas
border b(t=0,h1) {x=L;y=t;}; // droite
border f(t=L,0) {x=t;y=t*(h1-h)/L+h;}; // free surface
border d(t=h,0) {x=0;y=t;}; // gauche
int n=10;
mesh Th=buildmesh (a(L*n)+b(h1*n)+f(sqrt(L^2+(h-h1)^2)*n)+d(h*n));
plot(Th);
```



A free-boundary problem: modelling the water infiltration 2/2

The model is: find p and Ω such that:

$$\begin{cases} -\Delta p = 0 & \text{in } \Omega \\ p = y & \text{on } \Gamma_b \\ \frac{\partial p}{\partial n} = 0 & \text{on } \Gamma_d \cup \Gamma_a \\ \frac{\partial p}{\partial n} = \frac{q}{K} n_x & \text{on } \Gamma_f & (Neumann \\ p = y & \text{on } \Gamma_f & (Dirichlet) \end{cases}$$

where the input water flux is q = 0.02, and K = 0.5. The velocity u of the water is given by $u = -\nabla p$.



Modelling the water infiltration: the algorithm

We use the following fixed-point method: (with bad main B.C. Run:freeboundaryPB.edp) Let k = 0, $\Omega^k = \Omega$. For the first step, we forget the Neumann B.C. and we solve the problem: find p in $V = H^1(\Omega^k)$, such as p = y on Γ_b^k and Γ_f^k , and

$$\int_{\Omega^k} \nabla p \nabla p' = 0, \quad \forall p' \in V \text{ with } p' = 0 \text{ on } \Gamma_b^k \cup \Gamma_f^k$$

With the residual of the Neumann boundary condition, we build a domain transformation $\mathcal{F}(x,y) = [x, y - v(x)]$, where v is solution to: $v \in V$, such than v = 0 on Γ_a^k (bottom) and

$$\int_{\Omega^k} \nabla v \nabla v' = \int_{\Gamma_f^k} (\frac{\partial p}{\partial n} - \frac{q}{K} n_x) v', \quad \forall v' \in V \text{ with } v' = 0 \text{ sur } \Gamma_a^k$$

Remark: we can use the previous equation to evaluate

$$\int_{\Gamma^k} \frac{\partial p}{\partial n} v' = -\int_{\Omega^k} \nabla p \nabla v'$$

Modelling the water infiltration: implementation

```
The new domain is: \Omega^{k+1} = \mathcal{F}(\Omega^k).

Warning: if is the displacement is too large we can have triangle overlapping.

<sup>Vh</sup> u, v, uu, vv;

problem Pu(u, uu, solver=CG) = int2d(Th) ( dx (u) *dx (uu) +dy (u) *dy (uu))

+ on (b, f, u=y);

problem Pv(v, vv, solver=CG) = int2d(Th) ( dx (v) *dx (vv) +dy (v) *dy (vv))

+ on (a, v=0) + int1d(Th, f) (vv*((q/K) *N.y- (dx (u) *N.x+dy (u) *N.y)));

real errv=1;

while (errv>le-6) { j++;

Pu; Pv;

plot(Th, u, v, wait=0);

errv=int1d(Th, f) (v*v);
```

Here tricky code to take account the triangle overlapping

```
Th=movemesh(Th,[x,y-coef*v]); // calcul de la deformation
cout << "\n\n"<<j <<"-----_errv_=_" << errv << "\n\n";</pre>
```

Run:freeboundary.edp



5 Moving boundaries/ Eigenvalue problems / Parallel computing (Lesson 8)

- A free-boundary problem: modelling the water infiltration 1/2
- Eigenvalue problems



Computing eigenvalues and eigenvectors: an example

Find the first λ, u_{λ} such as:

$$a(u_{\lambda},v) = \int_{\Omega} \nabla u_{\lambda} \nabla v = \lambda \int_{\Omega} u_{\lambda} v = \lambda b(u_{\lambda},v)$$

Boundary conditions are imposed using exact penalization: we set to 1e30 = tgv the diagonal terms corresponding to locked degrees of freedom. Consequently, we impose Dirichlet boundary conditions only for the variational form of a and not for the variational form of b, because we compute eigenvalue of

$$\frac{1}{\lambda}v = A^{-1}Bv$$

Otherwise we can get spurious mode. FreeFem++ uses an Arpack interface:

int k=EigenValue(A,B,sym=true,value=ev,vector=eV);

Computing eigenvalues and eigenvectors: the program

```
real sigma = 0; // value of the shift
varf a(u1,u2) = int2d(Th)( dx(u1)*dx(u2) + dy(u1)*dy(u2) - sigma* u1*u2)
                    + on(1,2,3,4,u1=0); // Boundary condition
varf b([u1], [u2]) = int2d(Th)( u1*u2); // no Boundary condition
matrix A= a(Vh,Vh,solver=UMFPACK);
matrix B= b(Vh,Vh,solver=CG,eps=1e-20);
....
for (int i=0;i<k;i++)</pre>
{ u1=eV[i];
  real qq = int2d(Th)(dx(u1) * dx(u1) + dy(u1) * dy(u1));
  real mm= int2d(Th)(u1*u1) ;
  real err = int2d(Th)(dx(u1)*dx(u1) + dy(u1)*dy(u1) - (ev[i])*u1*u1);
  if(abs(err) > 1e-6) nerr++;
  if(abs(ev[i]-eev[i]) > 1e-1) nerr++;
  cout << ",----," << i<< "," << ev[i] << ",==," << eev[i] << ",err=." << err <<
      " --- "<<endl;
  plot (eV[i], cmm="Eigen_", Vector, "+i+", valeur, =" + ev[i] , wait=1, value=1, dim=3,
      fill=1);
```

Run:Lap3dEigenValue.edp

Run:LapEigenValue.edp Run:free-cvl-3d.edp

